

XVIII SPANISH-FRENCH SCHOOL JACQUES-LOUIS LIONS ABOUT NUMERICAL SIMULATION IN PHYSICS AND ENGINEERING Las Palmas de Gran Canaria, 25-29 June 2018



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Introduction: What is PILEDYN?

PILEDYN [1] is a software package based on a Boundary Element - Finite

Results: Pile group foundation impedances

The lateral K_h , vertical K_v , rocking K_r and cross K_c impedances (normalized stiffness and damping coefficients) calculated for a 3x3 pile group are shown, where L and dare the pile's length and diameter, α is the rake angle, s is the distance between piles, μ_s is the soil shear modulus and the c_s is the is the soil shear-wave velocity.

SēMA

- Element Method (BEM-FEM) for pile group linear dynamic analysis [2]
- Features: Able to obtain the pile group impedance in any topography (including stratified soils with several layers and rigid rocky beds).
- **BEM-FEM model main characteristics**:
 - Simplified, but rigorous!
 - This model has been used in many soil-structure interaction problems such as the determination of dynamic stiffnesses [3, 5] and seismic response of pile foundations [3, 6] and piled buildings [4].

Program structure

Pre-processor (MATLAB [7]): it generates input files for the solver. MESH2D [8] and gmsh [9] are used as meshing tools.





Figure: (Left) Mesh (1/4 symmetry). (Right) Impedances obtained with PILEDYN, with L/d = 15, $\alpha = 5$ and s/d = 5.

Forthcoming Research

- ► More features: pile group's envelopes and seismic response
- **Better GUI**: Octave suitable GUI, more input and output options

Figure: GUI appearance: pile group configuration menu.

- **Solver** (Fortran): uses BEM-FEM methodology to obtain the group response.
- **Post-processor** (MATLAB): from the response, rearranges the impedances.

Methodology

► FEM (Pile): Euler-Bernoulli beam (3 nodes finite element). If K and M are the stiffness (complex) and mass matrices, ω the circular frequency of the excitation, u_p the vector of nodal translations and rotations amplitudes and F the vector of nodal forces amplitudes and considering a pile with zero internal damping:

 $(\mathrm{K} - \omega^2 \mathrm{M}) \mathrm{u}^P = \mathrm{F} = \mathrm{F}^{ext} + \mathrm{Q} \mathrm{q}^P$

where \mathbf{F}_{ext} include the forces at the top \mathbf{F}_{top} and the axial force at the tip of the pile \mathbf{F}_p ; and \mathbf{F}_{eq} is the vector of the equivalent nodal forces from the pile-soil interaction, where \mathbf{Q} is the matrix that transforms nodal force components to equivalent nodal forces and \mathbf{q}^p the tractions along the pile-soil interface.

BEM (Soil): a continuum, homogeneous or zoned homogeneous, semi-infinite, isotropic, linear, viscoelastic medium. Only the free-surface and interfaces between strata, if any, are discretized. For a certain pile *j* and applying an unit load on pile *i*:

$$= p_i = p_i c_i c_j \qquad \sum_{p \in P_i} p_i p_j c_j \qquad \sum_{p \in P_i} p_j$$

References

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$\mathrm{Du}_b^{p_i} + \mathrm{H}_e^{p_i s} \mathrm{u}^s - \sum_{j=1} \mathrm{G}_e^{p_i p_j} \mathrm{q}^{s_j} + \sum_{j=1} \gamma_{b3}^{p_i j} \mathcal{F}_{p_j} = \mathbf{0}$

where \mathbf{u}_{b}^{pi} is the vector of nodal displacements at the bottom element nodes of the pile *i*, $\mathbf{H}_{e}^{p_{i}s}$ is the matrix obtained by integration over the boundary of the 3D elastodynamic fundamental solution times the shape functions of the boundary elements, \mathbf{u}^{s} is the vector of nodal displacements on the surface, n_{p} is the total number of piles, $\mathbf{G}_{e}^{p_{i}p_{j}}$ is the matrix obtained by integration over the pile-soil interface of the 3D elastodynamic fundamental solution times the interpolation functions, $\gamma_{b3}^{p_{ij}}$ is a 3 components vector that represents the contribution of the axial force $F_{p_{j}}$ at the tip of the jth pile, when the concentrated load is applied at the bottom element nodes of the pile *i*. D is the vector $1/8 \{0, 0, 3, 0, 0, 6, 0, 0, 1\}$.

► BEM - FEM coupling: Welded contact conditions at the pile-soil interface. $u^{s} = u^{p}; q^{s} = -q^{p} \rightarrow (K - \omega^{2}M) u^{p} - F_{p} + Q q^{s} = F_{top}$

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