

**XVIII SPANISH-FRENCH SCHOOL JACQUES-LOUIS LIONS ABOUT** NUMERICAL SIMULATION IN PHYSICS AND ENGINEERING Las Palmas de Gran Canaria, 25-29 June 2018

# Modelling of bedload sediment transport for weak and strong flow regimes - $N^{\circ} 2$ E.D. Fernández-Nieto<sup>1</sup>, G. Narbona Reina<sup>1</sup>, T. Morales de Luna<sup>2</sup>, C. Escalante<sup>3</sup> <sup>1</sup>Universidad de Sevilla. Dpto. Matemática Aplicada I

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Motivation

(Goal 2) The proposed bilayer bedload model

We focus on bedload transport where particles move by sliding or rolling over the bed. This phenomena is usually described by a Saint-Venant-Exner (SVE) model: a hydrodynamic component for water layer, coupled with a morphodynamic one for the sediment layer.

## Main drawbacks of the SVE:

× sediment discharge is defined empirically  $\times$  the mass conservation is not ensured

× no energy balance associated  $\times$  no gravitational effects

Goal n°1: Formal derivation of a SVE model from an appropriate asymptotic approximation of NS equations that counteracts these drawbacks. Goal n°2: To introduce a bilayer Saint-Venant model that generalizes the previous SVE model and converges to it in the bedload regime, i.e., when hydrodynamic time  $\ll$  morphodynamic time.

# (Goal 1) The proposed SVE model

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# Modelling framework



 $h_1$ : thickness of the water layer;  $h_2$ : thickness of the sediment layer;  $h_m$ : thickness of the mobile bed; **h**<sub>f</sub>: thickness of the fixed bed;

Submitted to ADWR. https://arxiv.org/abs/1711.03592 Bilayer model  $(M_2)$  $\partial_t h_1 + \operatorname{div}_x(h_1 u_1) = 0,$  $\partial_t(h_1u_1) + \operatorname{div}_x(h_1(u_1 \otimes u_1)) + gh_1\nabla_x(b+h_1+h_2) = -F,$  $(M_2) \langle \partial_t h_2 + \operatorname{div}_x(h_m u_m) = \mathbf{0},$  $\partial_t(h_m u_m) + \operatorname{div}_x(h_m(u_m \otimes u_m)) + gh_m \nabla_x(b + rh_1 + h_2)$  $= rF + \frac{1}{2}u_mT_m - (1-r)gh_msgn(u_m) \tan \delta,$ 

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# **★** Key point: **Friction term**

 $F = C_Q(u_1 - u_m)|u_1 - u_m|; \quad C_Q = \frac{1}{\alpha} \frac{h_1 h_m}{\vartheta(h_1 + h_m)}; \quad \alpha = \begin{cases} k_{\max} d_s & h_m \leq k_{\max} d_s \\ h_m & h_m > k_{\max} d_s \end{cases}$ 

#### **Advantages**

- + Avoid high numerical cost in approximation of gravitational effects
- **+** "Automatic" bedload regime behavior
- $\star$  (M<sub>2</sub>)  $\rightarrow$  (M<sub>1</sub>) when  $t_{hydro} \ll t_{morpho} (\Leftrightarrow u_{morpho} \ll u_{hydro})$
- $\star$  Non-hydrostatic effects may be incorporated.

### Numerical results

# $v_i = (u_i, w_i)$ : velocities; $\rho_i$ : densities; $r = \frac{\rho_1}{\rho_2} < 1$ .

# **Some considerations**

- ▷ Governing equations: 3D incompressible Navier-Stokes.
- ▷ "Fluids" with different physical properties.
- $\triangleright$  Flows with different behavior (hydrodynamic time  $\ll$  morphodynamic time). ▷ Interaction through friction laws.
- ▷ Arbitrary sloping beds.

# Asymptotic approach

- $\triangle$  Upper layer: Clear water.  $\bigtriangledown$  Lower layer: Bed load. **#** Different characteristic times
- $\Rightarrow$  shallow-water equation ( $\epsilon = H/L \ll 1$ )
- $\Rightarrow$  Reynolds equation (Pressure  $\gg$  convection)
- $\Rightarrow$  Multiscale analysis in time ( $t_{hydro} = \epsilon t_{morpho}$ )

# Resulting model $(M_1)$

# Test 1: Weak bedload transport (Dune)

Initial height:  $h_1 + h_2 = 1m$ ; left boundary condition  $q_1(t,0) = 1m^2/s^2$ .



# Test 2: Strong bedload transport (Dam-break problem)





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# Initial condition: $h_1 = 0.10m$ if x < 0 and $h_2 = 0.05m$

 $\partial_t h_f = -T_m$ 

Discharge obtained explicitly
 Associated dissipative energy

Gravitational effects Mass conservation

$$\frac{\tau_{\text{eff}}}{\rho_1} = \frac{\vartheta \, d_s \, \tau}{h_m \, \rho_1} - \frac{g d_s \vartheta}{r} \nabla_x (r h_1 + h_2 + b) \quad \text{and} \quad \theta_{\text{eff}} = \frac{|\tau_{\text{eff}}|/\rho_1}{(1/r - 1)g d_s}$$

**T** Generalization of MPM model for arbitrary sloping beds

$$\frac{q_b}{Q} = \operatorname{sgn}(\tau) \frac{8}{(1-\varphi)} (\theta - \theta_c)_+^{3/2} \longrightarrow \frac{q_b}{Q} = \frac{\operatorname{sgn}(\tau_{\text{eff}}) \frac{k_1}{1-\varphi} (\theta_{\text{eff}} - \theta_c)_+^{3/2}$$

	-0.6	-0.4	-0.2	0.0	0.2	0.4	0.6	0.8	1.0	-1.0	-0.5	0.0	0.5	1.0	
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#### Conclusions

The bilayer model gives promising results for weak and strong bedload transport regimes. It converges to the SVE model thanks to the definition of the friction terms. It is a good choice from a computational point of view for bedload problems.

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